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rigidity of a noncircular cross section as does the polar moment of inertia for a circular cross section. The term

$$\left(1 - 0.63 \frac{x}{y}\right) \frac{x^3 y}{3}$$

in Eq. (13-7) is a conservatively low approximation to the value of C for a rectangular section, assuming elastic behavior (see for example Reference 13.16). Since the value of C obtained by summing the values for each of the component rectangles making up a section will always be less than the theoretically correct value, it is appropriate to subdivide the cross section in such a way as to result in the highest possible value of C.

If a panel contains a beam parallel to the direction in which moments are being determined, the value of K_t obtained from Eq. (13-6) may lead to equivalent column stiffnesses which are too low. In such cases, the value of K_t given by Eq. (13-6) should be increased as follows:

$$K_{ta} = K_t \frac{I_{sb}}{I_s}$$

where K_{ta} is the increased torsional stiffness due to presence of parallel beam, I_s is the moment of inertia of a width of slab equal to the full width between center lines of panels, not considering the parallel beam, and I_{sb} is the moment of inertia of the width of slab used for the calculation of I_s, but including the contribution of that portion of the beam stem extending above or below the slab; the beam as defined in Section 13.1.5 does not apply in this calculation.

13.4.1.8 The use of only three-quarters of the full design live load for maximum-moment loading patterns is based on the fact that maximum negative and maximum positive live load moments cannot occur simultaneously and that redistribution of maximum moments is thus possible before failure occurs. This procedure, in effect, permits some local overstress under the full design live load if it is distributed in the prescribed manner, but still insures that the ultimate capacity of the slab system after redistribution of moment is not less than that required to carry the full design dead and live loads on all panels.

13.4.2 — This section corrects the negative design moments to the face of the supports. The correction is modified at an exterior support in order not to result in undue reductions in the exterior negative moment. Fig. 13-2 illustrates several equivalent rectangular supports for use in establishing faces of supports for design with nonrectangular supports.

13.4.5 — This section is a holdover from many previous Codes and is based on the principle that if two different methods are prescribed to obtain a particular answer, the Code should not require

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a value greater than the least acceptable value. Due to the long satisfactory experience with designs having total static design moments not exceeding those given by Eq. (13-2), it is considered that these values are satisfactory for design.

13.5—Slab reinforcement

The requirement that the center to center spacing of reinforcement be not more than two times the slab thickness applies only to the reinforcement in solid slabs, and not to that in joists or waffle slabs. This limitation is intended to insure slab action and reduce cracking and to provide for the possibility of loads concentrated on small areas of the slab.

13.5.2 — Bending moments in slabs at spandrel beams can be subject to great variation. If the beam should be built solidly into a masonry wall, the slab could be fixed. If no wall, the slab could be largely simply supported. This regulation provides for unknown conditions that might normally occur in a structure.

13.6—Openings in the slab system

See Commentary for Section 11.12 and Fig. 11-8.

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